

Comparison of Curve Number Calibration Methods

Langston Hughes II¹, Glenn Moglen²





INTRODUCTION/ABSTRACT

The Natural Resources Conservation Service curve number equation (Eq.1) estimates runoff volume based on precipitation depth and watershed-averaged storage, and an initial abstraction ratio, λ . This study compares two methods used to calibrate the storage parameter in this equation under varying values of λ . The primary purpose of this study is to examine the role of λ on the "goodness-of-fit" between model estimates and actual observations. The two methods used are 1) least-squares, and 2) median method. This study uses an Agricultural Research Service rainfall-runoff database (USDA 2019) of approximately 12,700 individual storm events drawn from 31 watersheds in 11 locations across the United States.

METHODS

Identify the 31 watershed files being used for observation. The 31 watersheds are in 11 locations across the United States and are provided by USDA. Each file contains hundreds of rainfall-runoff events (specific numbers appear in Table 1 below). With every event in the files there is a date, total precipitation depth measurement, and total runoff depth measurement.

| | Site/ Location | Watershed Area (ha) | Period of Record | Years | Number of Observations |
|---|-----------------------|------------------------|---------------------|-------|---------------------------|
| | Safford, AZ(1) | 210 | 1939-1969 | 31 | 110 |
| | Safford, AZ(2) | 276 | 1940-1969 | 30 | 104 |
| | Safford, AZ(3) | 309 | 1939-1969 | 31 | 90 |
| | Tifton, GA(1) | 33,400 | 1971-1980 | 10 | 297 |
| | Tifton, GA(2) | 1,570 | 1970-1980 | 11 | 441 |
| | Tifton, GA(3) | 1,590 | 1968-1980 | 13 | 552 |
| | Reynolds, ID(1) | 23,400 | 1963-1981 | 19 | 785 |
| | Reynolds, ID(2) | 3,180 | 1968-1981 | 14 | 492 |
| | Monticello, IL(1) | 33.2 | 1949-1981 | 33 | 222 |
| | Monticello, IL(2) | 18.4 | 1949-1981 | 33 | 344 |
| | Treynor, IA(1) | 60.7 | 1964-1986 | 23 | 602 |
| - | Treynor, IA(2) | 43.3 | 1964-1986 | 23 | 866 |
| | Treynor, IA(3) | 33.5 | 1964-1986 | 23 | 762 |
| ı | Hastings, NE(1) | 195 | 1940-1962 | 23 | 293 |
| | Hastings, NE(2) | 166 | 1939-1967 | 29 | 332 |
| | Hastings, NE(3) | 844 | 1938-1967 | 30 | 293 |
| | Hastings, NE(4) | 1,410 | 1939-1967 | 29 | 303 |
| | Albuquerque, NM(1) | 99.6 | 1939-1969 | 31 | 175 |

Table 1. Research watersheds used in this study.

Least-Squares Method (Numerical Optimization Program)

The relevant equation is provided by the Natural Resources Conservation Service (NRCS) rainfall-runoff equation(Mockus 1949; SCS 1975) shown below:

$$Q = \frac{(P - \lambda S)^2}{P + (1 - \lambda)S} \quad \text{if } P > \lambda S$$

Q is runoff (in inches), P is precipitation (in inches) and S is storage (in inches). P is the 24-hour rainfall depth. Curve number (CN) and S are related by Eq.2.

$$CN = \frac{1,000}{s+10} \tag{2}$$

Use a Numerical Optimization program to explore the rainfall-runoff S values. This program finds a storage(S) value that allows for the smallest "Z" as calculated in Eq.3.

$$Z = \sum_{i=1}^{n} (Q_{obs,i} - Q_i)^2$$
 (3)

 $Q_{obs,i}$ is the observed runoff from precipitation event, i, and Q_i are the modeled runoff for this same event.

Vary λ values (0.05 and 0.2) to assess differences in the goodness-of-fit with changing λ .

METHODS (Continued)

Median Curve Number Method (NEH Program)

Use a root-finding algorithm that reads as input a watershed file along with additional input parameters (rainfall threshold, λ value), and returns a median Curve Number value across all the rainfall-runoff observations where P exceeds the rainfall threshold in that watershed by solving for each individual storage(S) in Eq.1(Rallison and Cronshey 1979; SCS 1985).

Repeat for all watershed events while changing λ values (0.05 and 0.20)

Calculate S_e/S_y for both calibration methods and values of λ to identify which method and λ value performs best.

FINDINGS

| FINDINGS | | | | | | | | | | | | | |
|--------------------|------------------|-----------------------------------|---------------|---------------------|-------------------|------------------|------------------|----------|-------------|----------|----------|--|--|
| | | λ = 0.20 | | | | | | λ = 0.05 | | | | | |
| Least-Squares Meth | and (Numerical (| | Drogram/1 | 11 | | Least-Squares Me | ethod (Numerical | | n Program(1 |) | | | |
| Median CN Method | _ | Median CN Method (NEH Program)(2) | | | | | | | | | | | |
| Watershed Name | Watershed | | CNI/2V | Sa/Su/1\ | co/Sv/2\ | Watershed Name | | CN(1) | CN(2) | Se/Sy(1) | Se/Sy(2) | | |
| arizona | 45001 W-I | CN(1) 83.4 | CN(2) 80.7 | Se/Sy(1) 7 0.555 | se/Sy(2) 0.761 | arizona | 45001 W-I | 76.4 | 67.9 | | | | |
| arizona | 45002 W-II | 85.6 | | | | arizona | 45002 W-II | 80.3 | 74.9 | | | | |
| arizona | 45002 W-IV | 56.2 | | | | arizona | 45003 W-IV | 40.0 | 43.2 | | | | |
| georgia | 74002 W-TB | 58.8 | | | | georgia | 74002 W-TB | 40.0 | 38.0 | | | | |
| georgia | 74003 W-TN | 60.2 | | | | georgia | 74003 W-TN | 42.1 | 43.4 | | | | |
| georgia | 74004 W-TO | 62.5 | | | | | 74004 W-TO | 45.8 | 47.7 | 0.105 | | | |
| idaho | 68001 W-1 | 56.2 | | | 1.21 | idaho | 68001 W-1 | 40.0 | 32.3 | 0.361 | | | |
| idaho | 68003 W-3 | 58.1 | | | | idaho | 68003 W-3 | 40.0 | 29.3 | 0.440 | 1.28 | | |
| illinois | 61001 IA | 66.1 | | | | illinois | 61001 IA | 51.0 | 40.9 | 0.781 | 0.972 | | |
| illinois | 61002 IB | 60.8 | | | | illinois | 61002 IB | 45.0 | 36.3 | 0.847 | 0.931 | | |
| iowa | 71004 W-4 | 48.1 | | | 1.46 | iowa | 71004 W-4 | 40.0 | 40.1 | 0.120 | 1.16 | | |
| iowa | 71003 W-3 | 59.2 | | | | iowa | 71003 W-3 | 41.4 | 40.7 | 0.653 | 0.850 | | |
| iowa | 71002 W-2 | 73.1 | | | | iowa | 71002 W-2 | 61.5 | 47.1 | 0.650 | 1.09 | | |
| nebraska | W-3 | 78.9 | | | | nebraska | W-3 | 69.9 | 65.1 | 0.678 | 0.873 | | |
| nebraska | W-5 | 71.7 | | | | nebraska | W-5 | 59.0 | 53.2 | 0.637 | 0.748 | | |
| nebraska | W-8 | 70.2 | | | | nebraska | W-8 | 56.2 | 53.5 | 0.781 | 1.04 | | |
| nebraska | W-11 | 65.6 | | | | nebraska | W-11 | 50.4 | 46.6 | 0.746 | 0.915 | | |
| new mexico | 47001 W-I | 72.2 | 73.3 | 0.860 | 1.39 | new mexico | 47001 W-I | 54.5 | 54.8 | 0.754 | 1.02 | | |
| new mexico | 47002 W-II | 84.3 | 85.9 | 0.115 | 1.67 | new mexico | 47002 W-II | 76.4 | 74.5 | 0.109 | 1.41 | | |
| new mexico | 47003 W-III | 77.3 | 77.1 | 0.129 | 1.51 | new mexico | 47003 W-III | 64.1 | 64.0 | 0.116 | 1.23 | | |
| ohio | 26001 102 | 66.9 | 65.6 | 0.733 | 0.747 | ohio | 26001102 | 51.3 | 38.0 | 0.755 | 0.898 | | |
| ohio | 26003 129 | 69.2 | 70.4 | 0.837 | 1.07 | ohio | 26003129 | 53.9 | 47.2 | 0.680 | 0.973 | | |
| ohio | 26015 110 | 70.2 | 70.8 | 0.811 | 1.10 | ohio | 26015 110 | 55.8 | 47.2 | | | | |
| texas | 42010 W-10 | 78.8 | 82.1 | 0.866 | 1.15 | texas | 42010 W-10 | 70.6 | 74.2 | 0.828 | 0.743 | | |
| texas | 42011 Y | 70.0 | 72.1 | 0.770 | 1.22 | texas | 42011Y | 58.1 | 56.2 | 0.746 | 1.14 | | |
| texas | 42012 Y-2 | 74.4 | 76.9 | 0.703 | 1.02 | texas | 42012 Y-2 | 64.6 | 63.1 | 0.746 | 0.946 | | |
| vermont | 67002 W-2 | 74.2 | 74.7 | 0.114 | 1.72 | vermont | 67002 W-2 | 57.8 | 55.1 | 0.102 | 1.64 | | |
| vermont | 67003 W-3 | 71.7 | 73.5 | 0.976 | 1.23 | vermont | 67003 W-3 | 55.0 | 51.9 | | | | |
| virginia | 13008 B.C. | 63.6 | 74.7 | 0.134 | 1.62 | virginia | 13008 B.C. | 47.8 | 55.5 | | | | |
| virginia | 13009 P.C. | 68.6 | 74.7 | 0.125 | 1.24 | virginia | 13009 P.C. | 54.9 | 57.4 | | | | |
| virginia | 13010 L.W.C. | 63.7 | 70.5 | 0.955 | 1.29 | virginia | 13010 L.W.C. | 47.9 | 47.7 | | | | |
| | | | A | 0.517 | 1.00 | | | | Averages | 0.513 | 1.06 | | |

Table 2. Curve number and Standard Error (S_e/S_y) values corresponding to λ =0.20.

Tables 2&3. Red values indicate the lower S_e/S_y calculation corresponding to the λ value. The circled values in the λ =0.05 chart were the only calculation where the median approach was better than the least-squares approach.

 $\lambda = 0.05.$

Table 3. Curve number and Standard

Error (S_e/S_v) values corresponding to

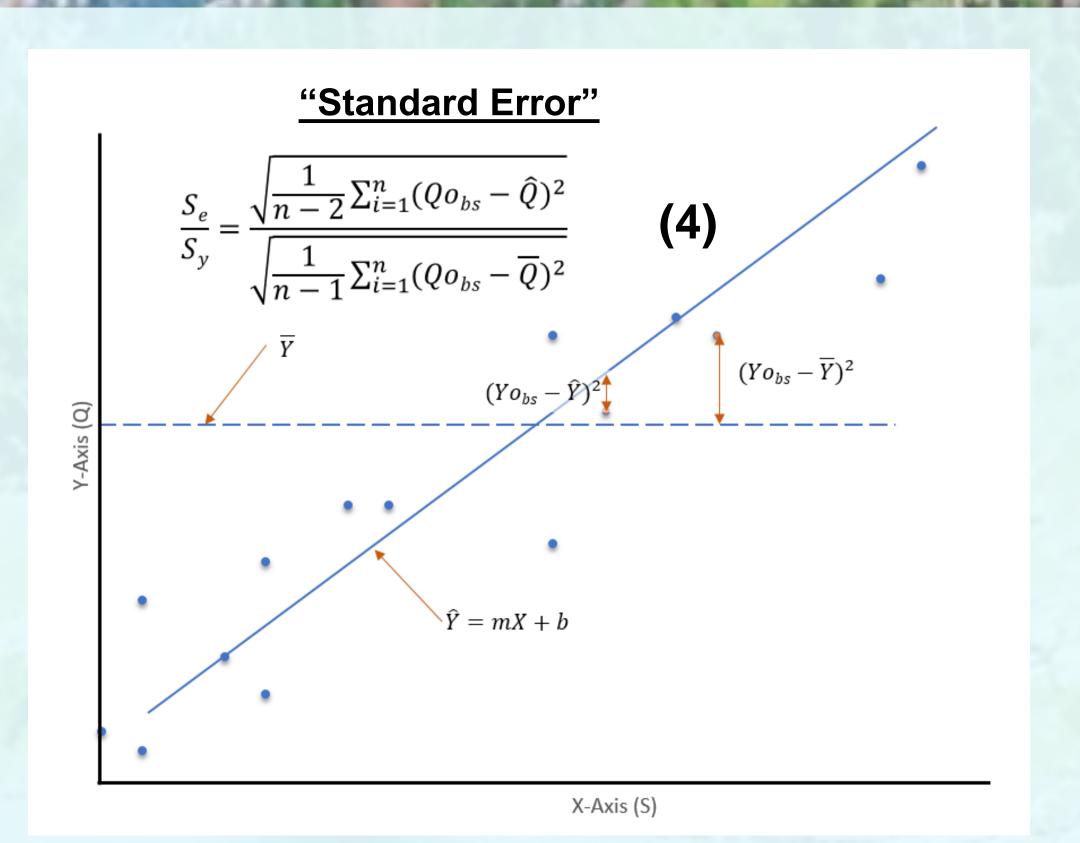


Figure 1. S_e/S_v formula and example graph.

APPLICATIONS

An accurate curve number(CN) calibration is important because the CN is used to estimate discharges which are used for hydrologic and hydraulic designs. Bridges, dams, and drainage pipes/culverts are examples of hydrologic and hydraulic designs.

CONCLUSIONS/ RESULTS

Results show that the calibrated storage for individual runoff events vary, that calibrated storage differs depending on the calibration method used, and that goodness-of-fit is both a function of the calibration method and λ . Goodness-of-fit measured by standard error is almost universally stronger when using the least-squares calibration method. Two values of λ were explored, (0.05 and 0.20) with 0.05 producing better goodness-of-fit in 8 out of 31 instances using least-squares method and 6 out of 31 instances using median method.

REFERENCES

Mockus, V. 1949. Estimate of total (and peak rates of) surface runoff for individual storms: Exhibit A. Appendix B, Interim Survey Rep., Grand (Neosho) River Watershed. Washington, DC: U.S. Dept. of Agriculture.

SCS (Soil Conservation Service). 1975. Urban hydrology for small watersheds. 1st ed. Washington, DC: SCS.

SCS. (1985). 'Hydrology', National Engineering Handbook, Supplement A, Section 4, Chapter 10, Soil Conservation Service, USDA, Washington, D.C.

Rallison, R.E., and Cronshey, R.C. (1979). "Discussion of "Runoff curve number with varying site moisture" by Hawkins, R.H." Journal of Irrigation and Drainage Division, ASCE, 105 (IR4), 439-441.

"Ag Data Commons Beta." ARS Water Database | National Agricultural Library, data.nal.usda.gov/dataset/ars-water-database.(accessed on 04/22/19)